Abstract—Large-scale exploitation of the tidal stream resource is likely to alter the regional hydrodynamics, but for practical extraction scenarios this effect is generally considered to be very small. However, since sediment transport is proportional to the cube of velocity, relatively small changes in the tidal currents could translate into large changes in the sediment dynamics. Here, we investigate this effect in relation to two oceanographic processes: tidal asymmetry and headland sand bank maintenance. Both of these processes have major practical significance. Tidal asymmetry/asymmetry is responsible for the large-scale long-term distribution of shelf sea sediment. Any tidal energy scheme which has the potential to alter this large-scale distribution could affect the supply of sediment feeding into natural coastal defence systems which remove energy from storm waves, such as beaches and offshore sand banks. Headlands are some of the most attractive regions for exploitation of the tidal stream resource. Any tidal energy scheme which could lead to changes in the morphodynamics of the associated headland sand banks could have implications for coastal flooding, due to changes in the wave distribution, including wave refraction and depth-induced wave breaking.

Index Terms—Tidal energy converter, tidal stream turbines, energy extraction, sediment dynamics, sand banks, tidal symmetry, Bristol Channel

I. INTRODUCTION

Tidal energy converter (TEC) devices operate by intercepting the kinetic energy in strong tidal currents (typically through a turbine unit). This intercepted energy is then converted to electrical energy through a power take-off system (e.g. an induction generator) and conditioned for dispatch to the electricity network. Theoretically, this is similar to the operation of a typical wind energy device. However, what is significantly different from the wind energy analogy is the environment that TEC devices operate in [1], and the potential for TEC devices to interact with their environment [2]. Although the key environmental variables likely to be impacted by TEC device operation have long been identified (e.g. collision risk, acoustic emissions, sediment dynamics and morphodynamics) [3], few of these impacts have yet been quantified [4]. This lack of progress is potentially slowing down the commercialisation and full-scale exploitation of the tidal stream resource.

Extracting energy from a tidal system will result in changes to the velocity structure, including an overall reduction in current speed over the larger area domain [2]. This reduction in current speed, even for relatively large TEC array extraction scenarios, is generally quite small. For example, in a tidal channel the impact of energy extraction on current speed $U$ becomes noticeable only when the energy extracted reaches around 10% of the available kinetic energy flux [5], a considerably large amount of energy to extract from a channel. More realistic extraction scenarios (typically 1% of the available kinetic energy) could therefore be perceived to have very little environmental impact. However, bed shear stress is a function of $U^2$. Therefore, small changes in the tidal currents could potentially lead to large changes in the resulting bed shear stress. Further, the transport of sediments is proportional to an even higher power of velocity than bed shear stress, e.g. total load transport by currents (bedload and suspended load) is a function of $U^{3.4}$ [6]. Therefore, relatively small changes to the residual flow field caused by exploitation of the tidal stream resource could have a significant influence on the residual sediment transport pathways.

Generally, sites suitable for TEC arrays are selected based on technical (e.g. water depth) and economic (e.g. mean kinetic power density) constraints (e.g. [7]). This research considers additional criteria which, if not addressed, could lead to negative environmental impacts. We examine the effects of TEC array operation on sediment dynamics and morphodynamics, with respect to two classical oceanographic processes: tidal asymmetry and headland sand bank maintenance.

II. TIDAL ASYMMETRY AND ENERGY EXTRACTION

Pingree and Griffiths [8] noted that the mean bed shear stress distributions associated with the principal lunar semi-diurnal ($M_2$) and quarter-diurnal ($M_4$) tides tend to be directed into bays and to diverge from $M_2$ amphidromic points, reaching maximum values at $M_4$ amphidromic points. They further suggested that the interaction between the $M_2$ and $M_4$ tides determines the direction of net sand transport around the British Isles. Regions of modelled bed stress divergence compared favourably with sediment bed load partings, based on observations of the seabed sediment composition around the British Isles [9] and in the Irish Sea [10], [11].
It is the phase relationship between the \( M_2 \) and \( M_4 \) tidal currents that leads to asymmetry [8], [12]. Although the combination of \( M_2 \) and \( M_4 \) tidal currents in Fig. 1a results in a distorted tide (Fig. 1b), the flood and ebb tides are equal in magnitude, as is the bed shear stress (based here on the assumption of a quadratic friction law). Hence, there is no residual sediment transport. By combining \( M_2 \) and \( M_4 \) tidal currents as in Fig. 2a, however, the flood tide is stronger than the ebb (Fig. 2b). Although there is no net residual flow, the integrated square of the velocity (\( U^2 \)) is greater during the flood. Hence, there is a residual bed shear stress and the net direction of sediment transport will be in the flood direction. In the case where the flood and ebb currents are equal

\[
2\phi_{M_2} = \phi_{M_4} + 90^\circ
\]

and where there is a maximum asymmetry

\[
2\phi_{M_2} = \phi_{M_4}
\]

where \( \phi_{M_2} \) and \( \phi_{M_4} \) are the phases (in degrees) of the \( M_2 \) and \( M_4 \) tidal currents, respectively. Hence, a plot of \( 2\phi_{M_2} - \phi_{M_4} \) quantifies tidal asymmetry, demonstrated in Fig. 3a for the tidal mean bed stress, \( \tau_0 \). Since bed level change is related to the divergence of the tidal residual bed shear stress, \( \tau_0 \), for 1D flow a plot of \( -\partial\tau_0/\partial x \) can be used to infer bed level change for spatial changes in \( 2\phi_{M_2} - \phi_{M_4} \) (Fig. 3b). Sediment will accumulate (i.e. positive bed level change) in regions of bed shear stress convergence (\( \partial\tau_0/\partial x < 0 \)) whereas sediment will be eroded from regions of bed shear stress divergence (\( \partial\tau_0/\partial x > 0 \)). Zero bed level change occurs in regions of maximum asymmetry (0° and 180°) whereas regions of symmetry (90° and 270°) are associated with the maximum magnitude of bed level change (since regions of tidal symmetry are either regions of sediment convergence or divergence).

When energy is extracted from a tidal system which exhibits varying magnitudes of tidal asymmetry, the effect on residual bed stress and the corresponding bed level change can be considered. In regions of tidal symmetry (90° and 270° in Fig. 3), energy extraction will have no influence on the large-scale morphodynamics of a tidal system since the residual bed stress is unaltered. In regions of maximum tidal asymmetry (0° and 180°), energy extraction is expected to have a maximum impact on the large-scale morphodynamics of a tidal system. In a relatively long tidal channel, different phase relationships between the \( M_2 \) and \( M_4 \) tidal currents will occur. The Bristol Channel, for example, has well-documented
Maximum tidal asymmetry and hence zero bed level change occurs at 0° and 180° from varying phase relationships between the M2 and M4 tidal currents. Maximum tidal asymmetry and hence zero bed level change occurs at 0° and 180°. Tidal symmetry and hence maximum bed level change occurs at 90° and 270°. (a) Tidal residual bed stress and (b) bed level change. These calculations are based on tidal current amplitudes of 1.0 and 0.25 m s⁻¹ for the M2 and M4 constituents, respectively, and C_D = 0.0025.

bed-stress divergence and convergence zones, which have been related to sediment transport and morphodynamics [11]. The hypothesis to be tested here is that in such a channel, energy extracted from regions of tidal asymmetry will have a much greater influence on sediment dynamics than energy extracted from regions of tidal symmetry.

A. Case Study: The Bristol Channel

The Bristol Channel (Fig. 4) separates South Wales from South West England and extends from the lower estuary of the River Severn to the Celtic Sea. For the purposes of this paper, however, the term Bristol Channel is taken to include the Severn Estuary. Depths in the outer region of the Bristol Channel are around 50 m, and an extensive system of narrow channels and tidal flats in the upper part of the estuary reduces the depth to less than 10 m upstream of Newport. With a mean spring range of 12.2 m and a mean neap range of 6.0 m at Avonmouth, the Bristol Channel has one of the highest semi-diurnal ranges in the world. From tidal analysis, the dominant constituents in the Bristol Channel are M2 and S2, but there is also a significant contribution from the M4 constituent [13]. The amplitudes of the S2 and M4 constituents at Avonmouth are 36% and 6%, respectively, of the M2. The interference of reflected and incoming tides in the Bristol Channel produces a partially progressive wave, with a phase difference between elevation and currents close to that for a standing wave [14]. A well known sediment bed load parting zone exists in the Bristol Channel to the west of Hinkley Point [11]. Numerical models that can accurately predict tidal asymmetry are crucial to the understanding of such zones, and hence for understanding large scale sediment dynamics.

There has been much speculation on the harvesting of tidal energy in the Bristol Channel. Most prominent are various tidal barrage schemes designed to capitalise on the energy resource contained in the large tidal range (e.g. [15]). Alternate tidal lagoon projects have also been proposed (e.g. [16]). In terms of the tidal stream resource, the Bristol Channel has been identified as accounting for around 2% of the total UK ‘extractable’ resource [17]. Although no large scale commercial scheme has yet been proposed for the area, Marine Current Turbines (MCT) installed a 300 kW rated experimental tidal stream turbine in the Bristol Channel, to the west of Hinkley Point. This test device, known as ‘Seaflow’, was operational in the years 2003-2009. In addition to the extreme tidal range and fast tidal currents in the Bristol Channel, the characteristics of the National Grid in the region are also encouraging for the development of new energy generation solutions. In most regions of the UK, the development of renewable energy is constrained by the lack of capacity available on the existing grid infrastructure. The south-west has been identified as a region of the UK where spare capacity currently exists both at the distribution and transmission level [18].

B. One-Dimensional Morphological Model

To test the hypothesis outlined above, a one-dimensional (1D) morphological model was developed and applied to the Bristol Channel, where large spatial variations in tidal asymmetry occur along the length of the channel. Previous studies have demonstrated that 1D models of the Bristol Channel are appropriate for reproducing the main features of the hydrodynamics [13], [19], [20]. The morphological model consisted of three components: hydrodynamic, sediment transport and bed level change models. The model is described fully in [21], hence only the governing equations and key points are included here.

Longitudinal variations in the time-varying free surface and velocity were calculated using the 1D sectionally-averaged continuity equation

$$\frac{\partial \eta}{\partial t} = -\frac{1}{B} \frac{\partial (AU)}{\partial x}$$

and momentum equation

$$\frac{\partial U}{\partial t} = -g \frac{\partial \eta}{\partial x} - \frac{C_D U |U|}{(h + \eta)^{4/3}} - \frac{P_x}{\rho U z \delta B (h + \eta)}$$

where \(\eta\) is the variation of the free surface from mean sea level, \(B\) is the channel width, \(A\) is the cross-sectional area of the...
channel, $U$ is the depth-averaged velocity, $C_D$ is the bottom friction coefficient, $h$ is the mean water depth, $\rho$ is water density, $\delta x$ is the model grid spacing and $g$ is gravitational acceleration. The final term in Eq. 4 is the energy extraction term, with power extracted ($P_x$ in Watts per grid point in the longitudinal direction) given as a function of current speed. The variation in the relationship between the power extracted and intercepted current speed is dependent upon the performance of the TEC technology adopted. As no device has yet reached commercialisation, the performance of individual devices is still the subject of development and debate. Douglas et al. have outlined basic technical specifications for a MCT ‘Seagen’ device [22]. Seagen is one of the more advanced technologies currently undergoing pre-commercialisation testing. Using the data provided in [22], the power curve presented in Fig. 5 was produced, based on the simplifying assumption of constant efficiencies across the performance range, and extending the rated velocity to 2.7 m s$^{-1}$ to produce a 1.5 MW rated device (a Seagen device has two rotors). This is an idealised representation of the potential performance of a TEC device, and is not expected to be wholly realistic - for instance, it would be desirable from a techno-economic perspective for the power coefficient ($C_p$) curve to reach a maximum below, rather than at, the rated velocity. At velocities below the cut-in speed, the turbine does not generate sufficient lift to rotate the drive train. At velocities which exceed the rated speed of the turbine, power output is constant. Since the energy extraction term is proportional to $U^2$, it is analogous to an additional bottom friction term [5].

Fig. 4. Location and bathymetry of the Bristol Channel. Contours are depths in metres relative to mean sea level and the filled circle north of Lundy is the offshore boundary of the one-dimensional morphological model.

Fig. 5. Idealised power curve per rotor used to parameterise energy extraction in numerical model (based on [22]).

Total instantaneous (bedload plus suspended load) sediment transport was calculated using the Soulsby-Van Rijn transport formulation [6]. 1D morphological development of the channel in the $x$-direction was modelled using (e.g. [23])

$$\frac{\partial z}{\partial t} = -\frac{1}{1 - p} \left\{ \frac{\partial q_t}{\partial x} \right\}$$

(5)

where $z$ is the bed level and $p$ is the bed porosity. The assumption was made that the bed was fully erodible for all
values of $x$.

C. Model Parameterisation

The morphological model was applied to a 155 km reach of the Bristol Channel, extending from an offshore location north of Lundy ($x = 0$) to the tidal limit (Fig. 4 and Fig. 6). The mean water depth and channel widths were extracted from a bathymetry dataset of the Irish and Celtic Seas which has a raw resolution of approximately 1 km [24]. This data was interpolated onto a longitudinal model grid spacing of $\delta x = 3167$ m (since the 155 km domain was divided into 49 cells, i.e. the model used 50 elevation points). The offshore model boundary was driven by elevation, using the $M_2$, $S_2$ and $M_4$ astronomical constituents extracted from an existing three-dimensional tidal model of the Irish Sea [25]. The $S_2$ constituent was included to simulate more realistic velocity magnitudes than the $M_2$ and $M_4$ constituents would have provided alone, and to include non-linear effects due to combining the dominant tidal constituents. However, since the model output was examined after an integer number of spring/neap cycles, the combination of $M_2$ and $S_2$ constituents did not contribute to tidal asymmetry (the $M_4$ contribution served this purpose). The upstream model boundary (at the tidal limit) was reflective. Detailed validation of the hydrodynamics is provided in [21], in terms of the amplitudes and phases of the $M_2$, $S_2$ and $M_4$ constituents at three locations along the channel. For such a relatively simple hydrodynamic model, the validation was very good, with mean errors of 13.6% in amplitude and 2.7% in phase.

To estimate the number of rotors ($n$) and the number of model grid cells required to accommodate a full-scale TEC array, the idealised power curve (Fig. 5) was used to parameterise the individual elements of a 250 MW tidal stream farm (approximately 168 devices, hence $n = 336$). Other than the availability of suitable tidal currents, practical/economic limitations restrict the exploitable resource available such as water depths in the range 25-45 m [1], and a minimum longitudinal turbine spacing of 15 device widths to reduce wake effects [26]. The overall (tip-to-tip) width of a Seagen device is 43 m, also used as the lateral spacing between devices. Hence, with a simple $12 \times 14$ rectangular array arrangement (number of longitudinal by lateral elements), this equates to a turbine farm with overall dimensions of 7568 m by 1161 m. Since the model grid spacing was 3167 m, power extraction for a 250 MW tidal stream farm can be represented by 168 Seagen devices distributed uniformly over 3 model grid cells.

The model was applied with a sediment median grain size of $d_{50} = 330$ $\mu$m (medium sand), based on in situ sampling over a large region of the estuary [27].

D. Model Results

The morphological model was applied to several simulations, each of duration 29.53 days (a lunar month) in order to remove any asymmetrical effects due to the combination of $M_2$ and $S_2$ constituents, assuming that the dynamics are largely linear. Initially, the power extraction term in Eq. 4 was switched off to provide a background bed level change over the modelled time period for a natural tidal channel. This resulted in maximum changes in bed level of magnitude 0.03 m (Fig. 6), corresponding with a diffuse zone of sediment divergence (bed load parting) between Ilfracombe and Hinkley ($x = 78$ km) and a diffuse zone of sediment convergence upstream of Hinkley ($x = 116$ km). Between these two regions of tidal symmetry was a zone of maximum tidal asymmetry ($x = 92$ km), where the net change in bed level was zero (compare with Fig. 3). Following this background simulation, energy was extracted from three strategic locations along the channel: a 250 MW tidal stream farm was centered on a region of sediment divergence (Fig. 6a), on a region of strong tidal asymmetry (Fig. 6b) and on a region of sediment convergence (Fig. 6c).

The results demonstrate that, regardless of the location of energy extraction, the magnitude of bed level change is damp-
enanced by the presence of a tidal stream farm (due to a general reduction in tidal velocity and hence net sediment transport). However, the location of energy extraction is important with regard to the magnitude of bed level change based on two main criteria: the magnitude of sediment transport at the point of extraction, and the degree of tidal asymmetry at the point of extraction. The first criterion is obvious, so the bed level change results have been normalised by the gross mean sediment transport at the point of energy extraction, averaged over the duration of the simulation ($Q_T$) to remove the effect of longitudinal variations in the magnitude of sediment transport at the point of energy extraction [21]. The normalised results are plotted in Fig. 7. The main finding is that when energy is extracted from a region of strong tidal asymmetry, the effect on the resulting bed level change is more pronounced (up to 29% difference from the natural tidal channel case) compared with energy extracted from regions of tidal symmetry (18% difference). For the three cases considered, this change was discernible at the bed load parting zone ($x = 78$ km) and in the region of strong tidal asymmetry ($x = 116$ km). Little change was discernible between the three cases in the region of the sediment convergence.

III. HEADLAND SAND BANK MAINTENANCE

Strong tidal flow past headlands and islands leads to the generation of large eddy systems, with an opposite sense of vorticity between the flood and ebb phases of the tide [28]. The outward-directed centrifugal force within each eddy is balanced by an inward-directed pressure gradient (PG). This leads to the inward movement of relatively coarse sediment near the bed (where the centrifugal force is weaker due to bed friction), and the formation of headland sand banks. At relatively high latitudes, the Coriolis force ($C$) leads to the formation of asymmetrical sand banks, with the cyclonic eddy generating a larger sand bank than the anticyclonic eddy. Grey shading indicates the location (and scale) of sand banks.

Reversing tidal flow

PG=Pressure Gradient
C=Coriolis Force
CF=Centrifugal Force

Fig. 8. Mechanism for headland sand bank formation, based on [29] and [30]. Reversing tidal flow past the headland leads to the generation of eddy systems with an opposite sense of vorticity between the flood and ebb phases of the tide. The outward-directed centrifugal force (CF) within each eddy is balanced by an inward-directed pressure gradient (PG). This leads to the inward movement of relatively coarse sediment near the bed (where the centrifugal force is weaker due to bed friction), and the formation of headland sand banks. At relatively high latitudes, the Coriolis force ($C$) leads to the formation of asymmetrical sand banks, with the cyclonic eddy generating a larger sand bank than the anticyclonic eddy. Grey shading indicates the location (and scale) of sand banks.

A. Two-Dimensional Morphological Model

The hydrodynamics for the headland case were generated by the three-dimensional (3D) POLCOMS model [39], formulated in spherical polar coordinates and using 20 terrain-following (sigma) layers in the vertical. Neglecting the baroclinic terms and barotropic meteorological effects (e.g. wind stress and atmospheric pressure), the depth-mean POLCOMS equations of motion are [39]

$$\frac{\partial \bar{u}}{\partial t} = f \bar{v} - \frac{g}{R \cos \phi} \frac{\partial \zeta}{\partial \chi} - \frac{F_B}{H} + NL_X \quad (6)$$

and

$$\frac{\partial \bar{v}}{\partial t} = -f \bar{u} - \frac{g}{R \sin \phi} \frac{\partial \zeta}{\partial \theta} + \frac{G_B}{H} + NL_\phi \quad (7)$$

where $\bar{u}$ and $\bar{v}$ are the depth-averaged eastward and northward velocities, respectively, $\zeta$ is water surface elevation relative to mean sea level (MSL), $H = h + \zeta$ is total water depth (where...
where $h$ is water depth relative to MSL, $\chi$ and $\phi$ are the eastwards and northwards coordinates, respectively, $f = 2\Omega \sin(\phi)$ is the Coriolis parameter (where $\Omega = 7.27 \times 10^{-5} \text{ s}^{-1}$ is the angular velocity of the Earth), $R$ is the radius of the Earth, $g$ is gravitational acceleration, $NL_x$ and $NL_\phi$ are the non-linear terms, and $F_B$ and $G_B$ are, respectively, the eastward and northward components of bed shear stress, calculated from

$$F_B, G_B = C_D(u_B, v_B) \sqrt{u_B^2 + v_B^2} \tag{8}$$

where $u_B$ and $v_B$ are the near-bed velocities, defined at a height $\delta$ above the bed, and $C_D$ is the drag coefficient (based on an assumed logarithmic velocity profile)

$$C_D = \left( \frac{\kappa}{\ln(\delta/z_0)} \right)^2, \quad C_D \geq 0.005 \tag{9}$$

where $\kappa = 0.41$ is Von Karman’s constant and $z_0$ is the roughness length (taken here as $10^{-3}$ m). After spinup, output from the hydrodynamic model was stored for each model cell every 15 minutes until completion of the simulation. The elevations and velocities output from the model were subsequently used as input to sediment transport formulae.

An additional bed friction source term was incorporated into the depth-mean POLCOMS equations (Eq. 6 and Eq. 7), with turbine characteristics parameterised from the power curve in Fig. 5. If we assume that the flow is rectilinear at the point of extraction (the ideal extraction scenario), and that the orientation of each device is optimised to make the maximum use of this flow direction, then power $P$ (in W per device) can be calculated as a function of the magnitude of the instantaneous velocity $U$. Therefore, the additional friction-type terms due to energy extraction ($p_x$ and $p_y$) to be added to Eq. 6 and Eq. 7 are

$$p_x = -nC_p \frac{P}{\rho A H} \cos(\theta) \tag{10}$$

and

$$p_y = -nC_p \frac{P}{\rho A H} \sin(\theta) \tag{11}$$

where $n$ is the number of turbines per model grid cell, $A$ is the area of each grid cell, $\rho$ is the density of seawater, and $\theta$ is the direction of the depth-averaged current.

Total instantaneous (bedload plus suspended load) sediment transport was again calculated using the Soulsby-Van Rijn transport formulation [6]. Two-dimensional (2D) morphological development was modelled using

$$\frac{\partial z}{\partial t} = -\frac{1}{1 - p} \left\{ \frac{\partial q_{tx}}{\partial x} + \frac{\partial q_{ty}}{\partial y} \right\} \tag{12}$$

where $q_{ti}$ is the total load transport of sediment in the $i$ direction.

Eq. 12 was implemented by reading the depth-averaged velocities at 15 minute intervals as output from the hydrodynamic model. These velocities were used to calculate the instantaneous sediment transport, and this was subsequently used as input to Eq. 12 to predict bed level change (i.e. the morphological timestep was 15 minutes). In each of the model runs, the duration of the simulation was less than one month, sufficient to examine the initial sedimentation/erosion of the bed. Since the change in bed level $\ll$ water depth over this time period, feedback between the evolving bed (Eq. 12) and the hydrodynamic flow field was not included. Further, it was assumed that an infinite supply of sediment was available for transport.

**B. Application to an Idealised Headland**

A channel of length 60 km was simulated, with a width of 50 km, and a constant depth of $h = 50$ m. A headland of length 15 km and width 3 km was located mid-way along the channel (Fig. 9), a configuration which has been demonstrated to produce headland sand banks [40]. Horizontal grid spacing was $\Delta x = \Delta y = 500$ m, and the open boundaries were driven by a normal component of depth-averaged velocity with an amplitude of $1 \text{ m s}^{-1}$ and period equal to the principal lunar semi-diurnal constituent ($M_2$). A no-slip boundary condition was applied to all coastlines. The duration of the simulations was 50 tidal cycles, sufficient to examine the initial sedimentation pattern. The highest velocities (amplitude $\sim 3.5 \text{ m s}^{-1}$) occur close to the headland and, after some trial simulations, a 3 km$^2$ region close to the tip of the headland was selected to site a TEC array (Fig. 9). Applying a lateral spacing of 3 turbine widths and a downstream spacing of 15 turbine widths to eliminate lateral and wake effects, respectively (e.g. [26]), a simple rectangular arrangement of $16 \times 1.5$ MW TEC devices can be accommodated within each of the 0.25 km$^2$ model grid cells, leading to a net (rated) TEC array of 288 MW. The sediment transport formulae were applied to a medium sand ($d_{50} = 300 \mu$m).

**C. Model Results**

In the absence of artificial energy extraction (i.e the baseline case), the modelled change in bed level for the idealised headland is given in Fig. 10a. From an initial flat bed, two sand banks have evolved with a horizontal length scale of around
Fig. 10. Modelled dominant sand bank formation for an idealised headland using $d_{50} = 300 \mu$m. Contours (in cm) show the magnitude of deposition after 50 tidal cycles, with varying contour intervals (labelled). (a) baseline case, (b) artificial energy extraction case, and (c) difference between (a) and (b). The dark shading indicates the location of the headland, and the light shading in (b) indicates the location of the TEC array.

7 km, one located on either side of the headland. Since the duration of this morphological simulation was 50 tidal cycles, this represents the initial sedimentation pattern, rather than equilibrium, hence the change in bed level is relatively small (around 4 cm). After including the parameterisation of the 288 MW TEC array in the hydrodynamic model, the resulting sedimentation pattern is qualitatively similar, but reduced in magnitude (Fig. 10b). The difference plot makes this clear (Fig. 10c), with a reduction in magnitude of deposition at the centre of the sand banks of around 30% compared with the baseline case. Note that due to the influence of the Coriolis force (Fig. 8) and the varying influence of friction between the flood and ebb phases of the tide (due to different water depths in the domain between flood and ebb) [31], there is slight asymmetry in sand bank formation, and hence in the influence of the TEC array. The results in Fig. 10 demonstrate that a full-scale TEC array located close to the tip of a headland could have an impact on the maintenance of the associated headland sand banks. These results suggest that TEC array developers should proceed with caution in the vicinity of headlands.

IV. DISCUSSION AND CONCLUSIONS

Results from 1D and 2D models have demonstrated that TEC array operation can have a significant impact on the morphodynamics of tidal systems. The magnitude of this impact depends upon the tidal asymmetry at the point of extraction. This supports the hypothesis presented in Section II: energy extracted from regions of strong tidal asymmetry will have a greater influence on large-scale sediment dynamics than energy extracted from regions of tidal symmetry. The resulting change in bed level was not restricted to the immediate vicinity of the TEC array, as would occur, for example, in the case of localised scour. In the model of the Bristol Channel, the change in morphodynamics due to TEC array operation was evident up to 50 km from the location of energy extraction.

Although the 1D model of the Bristol Channel demonstrates general impacts of TEC array operation on morphodynamics, the 2D headland sand bank model demonstrates more detailed impacts. Headland sand banks cause waves to refract and dissipate wave energy, and so have an important role in coastal processes, including coastal protection from storm waves. Our idealised model simulations indicate that a change in the deposition of sand on the headland sand banks of around 30% could occur during short-term TEC array operation. If the scale of this change is demonstrated to be significant compared to the natural range of inter-annual and inter-seasonal variability, then developers of TEC arrays would be advised to examine ways in which they could reduce the environmental impacts of TEC arrays sited in the vicinity of headlands.

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REFERENCES

